<u>Appendix</u> F

Preliminary Hydrology Memorandum



Date:	May 13, 2022	BKF Job Number:	C20212113-10
Deliver To:	Jenn Bodine, Smith Development		
From:	Mona Sadeghian, BKF Tom Morse, BKF		
Subject:	660 University Avenue Preliminary Hydrology Memorandum		

Purpose

The purpose of this memorandum is to provide a preliminary hydrology analysis for the redevelopment of the 660 University Avenue, 680 University Avenue, and 511 Byron Street parcels in Palo Alto, California. This analysis will estimate peak stormwater runoff from the site in both the existing and redeveloped conditions.

Background

The existing site is occupied by two buildings and a parking lot. The three lots will be merged into a single parcel totaling 0.52 acres. The site is bounded by Byron Street, University Avenue, Middlefield Road, Cardinal Dental, and a private residence. Drainage Area 1 is 0.52 acres which flows to University Avenue and eventually into the storm drain main in Middlefield Road.

The existing site conditions are shown in Exhibit A. The proposed redevelopment is shown in Exhibit B. Table 1 outlines the total impervious and pervious areas in the drainage area that are used in Attachment A: Runoff Calculations.

Table 1: Impervious and Pervious Are	as
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		Pervious Area	Impervious Area	% Pervious	% Pervious Change
Drainage Area 1	Existing Condition	0.05	0.47	10	+5
	Proposed Condition	0.08	0.44	15	

Methodology

This preliminary hydrology memorandum analyzes peak stormwater runoff from the site using the Rational Method for both the 10-year and 100-year storms in the existing and proposed conditions.

The Rational Method is defined by the formula:

- Q = C I A, where:
- Q = peak flow (cfs)
- C = runoff coefficient factor (unitless)
- I = rainfall intensity (in/hr) at the time of concentration
- A = area (acres)

This analysis uses runoff coefficients (C) as documented in figures 819.2A and 819.2B of the Caltrans Highway Design Manual, and are summarized in Table 2.

Table 2: Runoff Coefficients

Surface Type	С
Roof	0.9
Pavement	0.9
Landscape	0.4

The rainfall intensities for the 10-year and 100-year design storm are determined using IDF (intensityduration-frequency) Table: NOAA Atlas 14, Volume 6, and Version 2 for San Francisco International Airport, California. The rainfall intensity is based on the time of concentration (T_c), or the total time it takes rainfall to reach the analysis point along the longest path of travel. To simplify this preliminary analysis the existing time of concentration was conservatively set to be 10 minutes and the proposed time of concentration was set to be 15 minutes.

Analysis

Exhibit A shows the existing site topography and the existing breakdown of pervious and impervious areas. The runoff from the 0.52 acre site flows to University Avenue and eventually into the storm drain main in Middlefield Road.

Exhibit B shows the proposed site conditions and the proposed breakdown of pervious and impervious areas. The runoff will be directed to treatment planters prior to draining to the storm drain main in Middlefield Road. The proposed condition will match the existing condition.

The Rational Method was used to calculate peak stormwater runoff from the existing site and the proposed development for 10-year and 100-year storm events. The results are summarized in Tables 3 and 4. Calculation of existing condition and proposed development, peak stormwater runoff is shown in Attachment A: Runoff Calculations.



Table 3: Peak Stormwater Runoff, 10 Year Storm

	Existing Condition	Proposed Development	Difference	
	(CFS)	(CFS)	(CFS)	%
Drainage Area 1	0.80	0.62	-0.18	-22.1

Table 4: Peak Stormwater Runoff, 100 Year Storm

	Existing Condition	Proposed Development	Diffe	rence
	(CFS)	(CFS)	(CFS)	%
Drainage Area 1	1.27	0.99	-0.28	-21.9

Conclusion

Development of the project will not increase runoff to the City of Palo Alto storm drain system. Development of the site will increase landscape area, which will decrease peak stormwater discharge to the Middlefield Road storm drain main by about 22% during the 10-year storm 100-year storm as shown in Tables 3 and 4.

A detailed storm drain study will be completed with development of the project improvement plans documenting the final design and hydraulics.

Attachments:

- Exhibit A: 660 University Existing Drainage Areas
- Exhibit B: 660 University Proposed Drainage Areas
- Attachment A: Runoff Calculations









ATTACHMENT A: RUNOFF CALCULATIONS

660 UNIVERSITY - EXISTING RUNOFF

1. <u>Development Parameters</u>

Area Draining to storm drain main in Middlefield Road = 0.52 acres Assumed Time of Concentration (T_c) = 10 min

2. Time of Concentration & Rainfall Intensities

Rainfall intensities are taken from NOAA Atlas 14, Volume 6, and Version 2 for the San Francisco International Airport, California station. The rainfall intensity for the specified design storm over a duration equal to the time of concentration (10 min) is used. $I_{10} = 1.81$ in/hr $I_{100} = 2.87$ in/hr

3. <u>Runoff Coefficient Calculations</u>

Runoff coefficients (C) are documented in figures 819.2A and 819.2B from the Highway Design Manual, and are summarized below:

 Table 1: Runoff Coefficients

Surface Type	С
Roof	0.9
Pavement	0.9
Landscape	0.4

In the existing conditions, there are a mix of impervious and pervious areas in the drainage basins. Drainage Area 1 contains 0.52 acres, and includes a total pervious area of 0.05 acres and a total impervious area of 0.47 acres. The runoff coefficients for the site can be calculated using a weighted average of the runoff coefficients for pavement and landscape. $C_{\text{Drainage Area 1}} = (0.05 / 0.52) \times (0.4) + (0.47 / 0.52) \times (0.9) = 0.85$

4. <u>Runoff (Q) in cubic feet per second</u>

Q = CIA, where:

Q = Peak Discharge (cubic feet of fluid per second)

- C = Runoff Coefficient
- I = Rainfall Intensity (inches per hour) at the time of concentration

A = Watershed Area (acres)

Drainage Area 1:

10 Year = $(0.85) \times (1.81 \text{ in/hr}) \times (0.52 \text{ acres}) = 0.80 \text{ cfs}$ 100 Year = $(0.85) \times (2.87 \text{ in/hr}) \times (0.52 \text{ acres}) = 1.27 \text{ cfs}$

ATTACHMENT A: RUNOFF CALCULATIONS

660 UNIVERSITY - PROPOSED RUNOFF

1. <u>Development Parameters</u>

Area Draining to storm drain main in Middlefield Road = 0.52 acres Assumed Time of Concentration (T_c) = 15 min

2. Time of Concentration & Rainfall Intensities

Rainfall intensities are taken from NOAA Atlas 14, Volume 6, and Version 2 for the San Francisco International Airport, California station. The rainfall intensity for the specified design storm over a duration equal to the time of concentration (15 min) is used. $I_{10} = 1.46$ in/hr $I_{100} = 2.32$ in/hr

3. <u>Runoff Coefficient Calculations</u>

Runoff coefficients (C) are documented in figures 819.2A and 819.2B from the Highway Design Manual, and are summarized below:

 Table 1: Runoff Coefficients

Surface Type	С
Roof	0.9
Pavement	0.9
Landscape	0.4

In the proposed conditions, there are a mix of impervious and pervious areas in the drainage basins. Drainage Area 1 contains 0.52 acres, and includes a total pervious area of 0.08 acres and a total impervious area of 0.44 acres. The runoff coefficients for the site can be calculated using a weighted average of the runoff coefficients for pavement and landscape. $C_{Drainage Area 1} = (0.08 / 0.52) \times (0.4) + (0.44 / 0.52) \times (0.9) = 0.82$

4. <u>Runoff (Q) in cubic feet per second</u>

Q = CIA, where:

Q = Peak Discharge (cubic feet of fluid per second)

- C = Runoff Coefficient
- I = Rainfall Intensity (inches per hour) at the time of concentration

A = Watershed Area (acres)

Drainage Area 1:

10 Year = $(0.82) \times (1.46 \text{ in/hr}) \times (0.52 \text{ acres}) = 0.62 \text{ cfs}$ 100 Year = $(0.82) \times (2.32 \text{ in/hr}) \times (0.52 \text{ acres}) = 0.99 \text{ cfs}$



Santa Clara Valley *Urban Runoff* Pollution Prevention Program Date Form Completed: Trini Inouye

05/13/22

Provision C.3 Data Form

Which Projects Must Comply with Stormwater Requirements?

All projects that create and/or replace 10,000 sq. ft. or more of impervious surface on the project site must fill out this worksheet and submit it with the development project application.

All restaurants, auto service facilities, retail gasoline outlets, and uncovered parking lot projects (stand-alone or part of another development project, including the top uncovered portion of parking structures) that create and/or replace **5,000 sq. ft.** or more of impervious surface on the project site must also fill out this worksheet.

Interior remodeling projects, routine maintenance or repair projects such as re-roofing and re-paving, and single family homes that are not part of a larger plan of development are **NOT** required to complete this worksheet.

What is an Impervious Surface?

An impervious surface is a surface covering or pavement that prevents the land's natural ability to absorb and infiltrate rainfall/stormwater. Impervious surfaces include, but are not limited to rooftops, walkways, paved patios, driveways, parking lots, storage areas, impervious concrete and asphalt, and any other continuous watertight pavement or covering. Pervious pavement, underlain with pervious soil or pervious storage material (e.g., drain rock), that infiltrates rainfall at a rate equal to or greater than surrounding unpaved areas OR that stores and infiltrates the water quality design volume specified in Provision C.3.d of the Municipal Regional Stormwater Permit (MRP), is not considered an impervious surface.

For More Information

For more information regarding selection of Best Management Practices for stormwater pollution prevention or stormwater treatment contact:______

1. Project Information

Project Name: 660	0 University Ave	APN #	120-03-042, 120-03-043, 120-03-044			
Project Address:	roject Address: 511 Byron St, 660 University Ave, 680 University Ave/500 Middlefield Rd, Palo Alto, CA 94301					
Cross Streets:	Cross Streets: Byron St, University Ave, Middlefield Road					
Applicant/Develo	per Name: Smith Development					
Project Phase(s):	1 of 1 Engineer: BKF Enginee	15				
Project Type (Ch	eck all that apply): I New Development	🔽 Rede	velopment			
Private F	Public					
🗹 Residential	Commercial 🔲 Industrial 🗹 Mixed U	se 🔲 Ins	stitutional			
Restaurant	Restaurant 🔲 Uncovered Parking 🔲 Retail Gas Outlet 🔲 Auto Service (SIC code)					
(5013-5014, 5541, 7532-7534, 7536-7539)						
Project Description: Mixed-use building with basement parking on a 0.52 acre lot						

Project Watershed/Receiving Water (creek, river or bay):_San Francisquito Creek

2. Project Size

a. Total Site Area: 0.52 acres		b. Total Site Area Disturbed: 0.52 (including clearing, grading, or excavating)			acres
Site Totals	Total Existing (Pre-project) Area (ft ²)	Existing Area Retained ¹ (ft ²)	Existing Area Replaced ² (ft ²)	New Area Created ² (ft ²)	Total Post- Project Area (ft²)
c. Total Impervious Area (IA)	20,390	0	19,118	0	19118
d. Total new and replaced impervious area			19,1	18	
e. Total Pervious Area (PA) ³	2,136				3,408
f. Total Area (IA+PA)	22,526				22,526
g. Percent Replacement of IA in Redevelopment Projects: (Existing IA Replaced + Existing Total IA) x 100% 93.76 %					

3. State Construction General Permit Applicability:

- a. Is #2.b. equal to 1 acre or more?
 - Yes, applicant must obtain coverage under the State Construction General Permit (see <u>https://www.waterboards.ca.gov/water_issues/programs/stormwater/construction.html</u>)
 - No, applicant does not need coverage under the State Construction General Permit.

4. MRP Provision C.3 Applicability:

- a. Is #2.d. equal to 10,000 sq. ft. or more, or 5,000 sq. ft. or more for restaurants, auto service facilities, retail gas outlets, and stand-alone uncovered parking?
 - Yes, C.3. source control, site design and treatment requirements apply
 - No, C.3. source control and site design requirements may apply check with local agency
- b. For redevelopment projects, is #2.g. equal to 50% or more?
 - Yes, C.3. requirements (site design and source control, as appropriate, and stormwater treatment) apply to the entire site
 - INO, C.3. requirements only apply to the impervious area created and/or replaced
- c. Does the project create and/or replace 5,000 sf or more of impervious surface parking?
 - Yes, C.3. requirements may apply to the entire site check with local agency No

5. Hydromodification Management (HM) Applicability:

- a. Does the project create and/or replace one acre or more of impervious surface AND is the total post-project impervious area greater than the pre-project (existing) impervious area?
 - Yes (continue)
 Yes (continue)
 No exempt from HM, go to page 3
- b. Is the project located in an area of HM applicability (green area) on the HM Applicability Map? (<u>www.scvurppp.org/hmp-map</u>)
 - Yes, the project must implement HM requirements
 - No, the project is exempt from HM requirements

¹ "Retained" means to leave existing IA in place. An IA that goes through maintenance (e.g., pavement resurfacing/slurry seal/grind), but no change in grade is considered "retained".

² The "new" and "replaced" IA are based on the total area of the site and not specific locations on site. For example, impervious parking created over a pervious area is not "new" IA, if an equal amount of pervious area replaces IA somewhere else on the site. Constructed IA on a site that does not exceed the total pre-project IA will be considered "replaced" IA. A site will have "new" IA only if the total post-project IA exceeds the total pre-project IA (total post-project IA – total pre-project IA = New IA).

³ Include bioretention areas, infiltration areas, green roofs, and pervious pavement in PA calculations.

6. Selection of Specific Stormwater Control Measures:

o. Selection of specific storing	vater Control Measures.	
Site Design Measures	Source Control Measures	Treatment Measures
Minimize land disturbed (e.g., protect trees and soil)	■ Wash area/racks, drain to sanitary sewer ⁵	None (all impervious surface drains to self-
Minimize impervious surfaces (e.g., reduction in post-project impervious surface)	 Covered dumpster area, drain to sanitary sewer⁶ Sanitary sewer connection 	IID Treatment Bioretention area
 Minimum-impact street or parking lot design (e.g., parking on top of or under buildings) 	or accessible cleanout for swimming pool/spa/fountain ⁶ Beneficial landscaping (minimize irrigation, runoff,	 Flow-through planter Tree Well Filter or Trench with bioretention soils Rainwater harvest/use (e.g., cistern or rain barrel for
 Cluster structures/ pavement Disconnected downspouts (direct runoff from roofs, sidewalks, patios to landscaped areas) 	 pesticides and fertilizers; promotes treatment) Outdoor material storage protection Covers, drains for loading 	designated use, sized for C.3.d treatment) Infiltration trench Infiltration well/dry well Subsurface Infiltration
 Pervious pavement Green roof 	docks, maintenance bays, fueling areas	System (e.g. vault or large diameter conduit over drain rock)
Other self-treating ⁴ area (e.g., landscaped areas)	Maintenance (pavement sweeping, catch basin cleaning, good	Other
Self-retaining ⁴ area	housekeeping)	Non-LID Treatment Methods
 Interceptor trees³ Rainwater harvesting and use (e.g., rain barrel, cistern 	Storm drain labeling Other	 Proprietary high flow rate tree box filter⁷ Proprietary high flow media
for designated use) ³ Preserved open space: ac. or sq. ft. (circle one)		filter (sand, compost, or proprietary media) ⁷ Vegetated filter strip ⁸ Extended detention basin ⁸
Protected riparian and wetland areas/buffers (Setback from top of bank: ft.)		 Vegetated swale⁸ Other
Other Elaw Duration Controls for Hudge	modification Management (HM)	
Extended Undergro Detention basin vault	ound tank or Discretention with out control	let Other
⁴ See SCVURPPP C3 Handbook for defi	initions. https://sevurppp.org/2016/06/20/e-3-stor	mwater-handbook-iune-2016/

⁵ Optional site design measure; does not have to be sized to comply with Provision C.3.d treatment requirements.
⁶ Subject to sanitary sewer authority requirements.

7 These treatment measures are only allowed if the project qualifies as a "Special Project".

8 These treatment measures are only allowed as part of a multi-step treatment process (i.e., for pretreatment).

Stormwater Treatment Measure (STM)	Hydraulic Sizing Criteria Used*
Flow-through planter	3
	Choose from list
	Choose from list
	Choose from list

7. Stormwater Treatment Measure (STM) Sizing for Projects with Treatment Requirements

*Key: 1a: Volume - WEF Method

1b: Volume - CASQA BMP Handbook Method

2a: Flow - Factored Flood Flow Method

2b: Flow - CASQA BMP Handbook Method

2c: Flow - Uniform Intensity Method

3: Combination Flow and Volume Design Basis

8. Alternative Certification: Was the treatment system sizing and design reviewed by a qualified thirdparty professional that is not a member of the project team or agency staff?

Yes No Name of Third-party Reviewer

9. Operation & Maintenance Information

A. Property Owner's Name Smith Development B. Responsible Party for Stormwater Treatment/Hydromodification Control O&M:

- a. Name: Jenn Bodine
 - b. Address: 862 Villa Street, Suite G, Mountain View, CA 94041
- c. Phone/E-mail: jenn@smithdevelopment.com

This section to be completed by Municipal staff.

O&M Responsibility Mechanism

Indicate how responsibility for O&M is assured. Check all that apply:

O&M Agreement

Other mechanism that assigns responsibility (describe below):

This section to be completed by Municipal staff (Note: This is an optional section that agencies should modify per their internal review and tracking process.) Reviewed By:

Community Development Department	Public Works Department
Planning Division:	Engineering:
Building Division:	Other (Specify):
Return form to:	Data entry performed by: